## DEPARTMENT OF CIVIL ENGINEERING

## LAB MANUAL

## STRUCTURAL ANALYSIS

## NATIONAL INSTITUTE OF TECHNOLOGY,SRINAGAR

## Vision of the Institute

To establish a unique identity of a pioneer technical Institute by developing a high-quality technical manpower and technological resources that aim at economic and social development of the nation as a whole and the region in particular keeping in view the global challenges.

## Mission of the Institute

M1. To create a strong and transformative technical educational environment in which fresh ideas, moral principles, research and excellence nurture with international standards.

M2. To prepare technically educated and broadly talented engineers, future innovators and entrepreneurs, graduates with understanding of the needs and problems of the industry, the society, the state and the nation.

M3. To inculcate the highest degree of confidence, professionalism, academic excellence and engineering ethics in budding engineers.

## VISION OF THE DEPARTMENT

To nurture Civil engineers with passion for professional excellence, ready to take global challenges and to serve the society with high human values.

## MISSION STATEMENT OF THE DEPARTMENT

(1) To provide facilities and infrastructure for academic excellence in the field of Civil engineering.
(2) To inculcate in the student the passion for understanding professionalism, ethics, safety, sustainability and then actively contribute in the society.
(3) To nurture creativity and encourage innovative solutions to real life challenging problems in Civil engineering students.
(4) To prepare student for lifelong learning in global perspective.

## PROGRAM EDUCATIONAL OBJECTIVES

PEO1: To prepare students to get employment, profession and/or to pursue postgraduation and research in Civil engineering discipline in particular and allied engineering fields in general.

PEO2: To prepare students to identify and analyse Civil engineering problems in an iterative approach that involves defining, quantifying, testing and review of the identified problem.

PEO3: To prepare students to plan, organize, schedule, execute and communicate effectively as an individual, a team member or a leader in multidisciplinary environment.

PEO4: To provide the students, an academic environment that makes them aware of excellence in field of Civil Engineering and enables them to understand significance of lifelong learning in global perspective.

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## Experiment No. 1

## Flexural rigidity

Aim: - To find the value of flexural rigidity (EI) for a given beam and compare it with theoretical value

Apparatus: - Elastic Properties of deflected beam, weight's, hanger, dial gauge, scale, and Verniar caliper.

Formula: - (1) Central upward deflection, $\mathrm{y}=\mathrm{W} \cdot \mathrm{a} . \mathrm{L}^{2} / 8 \mathrm{y}$
(2) $\mathrm{EI}=\mathrm{W} . \mathrm{a} . \mathrm{L}^{2} / 8 \mathrm{y}$
(3) Also it is known that EI for beam $=\mathrm{E} \times \mathrm{bd}^{3} / 12$

Diagram:-


Theory: - For the beam with two equal overhangs and subjected to two concentrated loads W each at free ends, maximum deflection y at the centre is given bycentral upward deflection.Central upward deflection, $y=W . a \cdot L^{2} / 8 E I$
Where,
a = length of overhang on each side
$\mathrm{W}=$ load applied at the free ends
$\mathrm{L}=$ main span
$\mathrm{E}=$ modulus of elasticity of the material of the beam
I = moment of inertia of cross section of the beam
$\mathrm{EI}=\mathrm{W} . \mathrm{a} . \mathrm{L}^{2} / 8 \mathrm{y}$
It is known that, EI for beam $=\mathrm{E} \times \mathrm{bd}^{3} / 12$
Where, $b=$ width of beam
d = depth of beam

## Procedure: -

i) Find $b$ and $d$ of the beam and calculate the theoretical value of EI by Eq. (3).
ii) Measure the main span and overhang span of the beam with a scale.
iii) By applying equal loads at the free end of the overhang beam, find the central deflection $y$.
iv) Repeat the above steps for different loads.

Observation: - 1) Length of main span, $\mathrm{L}(\mathrm{cm})=$
2) Length of overhang on each side, $a(\mathrm{~cm})=$
3) Width of beam, $b(\mathrm{~cm})=$
4) Depth of beam, $d(\mathrm{~cm})=$
5) Modulus of elasticity, $\mathrm{E}(\mathrm{kg} / \mathrm{cm} 2)=2 \times 10^{6}$

## Observation Table:-

| Sr. |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| No | . Equal <br> loads at the <br> two <br> ends (kg) | Dial gauge <br> reading at <br> the <br> midspan of <br> beam(cm) | EI from <br> Eq. (3) | EI from <br> $\operatorname{Eq}(2)$ |
|  |  |  |  |  |
|  |  |  |  |  |
| . |  |  |  |  |
|  |  |  |  |  |

Calculation: - Average values of EI from observation $=$ $\qquad$ cm4

Average values of EI from calculation $=$ $\qquad$ .cm ${ }^{4}$

Result: - Flexural rigidity (EI) is found same theoretically and experimentally.
Precaution:- 1. Measure the center deflection y very accurately.
2. Ensure that the beam is devoid of initial curvature.
3. Loading should be within the elastic limit of the materials.

## Question:-

1. What is the unit of flexural rigidity?
2. Which types of beam are used in defected beam apparatus?
3. Define the size of beam which is used is used in defected beam apparatus.
4. What is the difference $\mathrm{b} / \mathrm{w}$ flexural rigidity and flexural stiffness?
5. What is flexural rigidity?

## Experiment No. 2

## Maxwell's reciprocal theorem

Aim: - To verify clerk Maxwell's reciprocal theorem
Apparatus: - Clerk Maxwell's Reciprocal Theorem apparatus, Weight's, Hanger, Dial Gauge, ScaleVerniar caliper.

## Diagram:-



## Theory: -

Maxwell theorem in its simplest form states that deflection of any point $A$ of any elasticstructure due to load $P$ at any point $B$ is same as the deflection of beam due to same loadapplied at A. It is, therefore easily derived that the deflection curve for a point in a structure is the sameas the deflected curve of the structure when unit load is applied at the point for which theinfluence curve was obtained.

## Procedure: -

i) Apply a load either at the centre of the simply supported span or at the free end of thebeam, the deflected form can be obtained.
ii) Measure the height of the beam at certain distance by means of a dial gauge before andafter loading and determine the deflection before and after at each point separately. iii) Now move a load along the beam at certain distance and for each positions of the load, the deflection of the point was noted where the load was applied in step1.This deflection should be measured at each such point before and after the loading, separately.
iv) Plot the graph between deflection as ordinate and position of point on abssica the plot for graph drawn in step2 and 3.These are the influence line ordinates for deflection of thebeam.

## Observation Table:-

| Distance <br> from the <br> pinned <br> end | Load at central point/ <br> cantilever end |  | Deflection <br> of various <br> points | Load moving along <br> beam <br> (mm)2-3 |  | Deflection <br> of various <br> points |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Beam <br> unloaded <br> Dial gauge <br> reading <br> $(\mathrm{mm})^{2}$ | Beam <br> loaded <br> Dial <br> gauge <br> reading <br> $(\mathrm{mm})^{3}$ | Beam <br> unloaded <br> Dial gauge <br> reading <br> $(\mathrm{mm})^{5}$ | Beam <br> unloaded <br> Dial gauge <br> reading <br> $(\mathrm{mm})^{5}$ | Beam <br> loaded <br> Dial gauge <br> reading <br> $(\mathrm{mm})^{6}$ |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

Result: - The Maxwell reciprocal theorem is verified experimentally and analytically.
Precaution: - (i) Apply the loads without any jerk.
(ii) Perform the experiment at a location, which is away from any
(iii) Avoid external disturbance.
(iv) Ensure that the supports are rigid.

## Question:-

1. What is the Maxwell's reciprocal theorem or define the Maxwell's reciprocal theorem?
2. What are the purpose of providing dial gauge and magnetic base in the apparatus?
3. Maxwell reciprocal theorem in structural analysis can be applied in-
A. all elastic structures
B. plastic structure
C. symmetrical structures only
D. prismatic element structure only
4. What is the difference $B / W$ Maxwell's reciprocal theorem and betties

## Experiment: 3

## Three hinged arch

## Aim:

To study the behaviour of a three hinged arch experimentally for the horizontal and vertical displacement of the roller end for a given system of loading and to compare the same with the results obtained by analytical calculations.

## Apparatus:

Three hinged arch apparatus, weights, scale, dial gauge, etc.

## Theory:

A three hinged arch is a determinate structure with the axial thrust assisting in maintaining the stability. The horizontal thrust $H$ in the arch for a number of loads can be obtained as follows


The reactions $V_{A}$ and $V_{B}$ are calculated using the following equations:

$$
\begin{aligned}
V_{A} & =\frac{\left[W_{1}\left(L-a_{1}\right)+W_{2}\left(L-a_{2}\right)+W_{3}\left(L-a_{3}\right)+W_{4}\left(L-a_{4}\right)\right]}{L} \\
V_{B} & =\frac{\left[W_{1} a_{1}+W_{2} a_{2}+W_{3} a_{3}+W_{4} a_{4}\right]}{L} \\
H_{A}+H_{B} & =0 \\
V_{A}+V_{B} & =W_{1}+W_{2}+W_{3}+W_{4}
\end{aligned}
$$

Take Moment about the hinge $C$

## Procedure:

1. Use lubricating oil at the roller end of the arch so as to have a free movement of the roller end.
ii. Balance the scif-weight of the arch by placing load on hanger for horizontal thrust until the equilibrium conditions is obtained. Under this condition, the roller end of the arch has a tendency to move inside on tapping the table. Note down the load in kg.
iii. Place a few loads on the arch in any chosen positions. Balance these by placing additional weights on the hanger for horizontal thrust. The additional weights on the thrust hanger give the experimental value of the horizontal thrust.

## Observation:

Span of the arch,
Central rise,
Initial load on the thrust hanger for balancing,

$$
\begin{array}{ll}
L & = \\
h & = \\
& =
\end{array}
$$

| $\begin{gathered} \text { S. } \\ \text { No. } \end{gathered}$ | Load Applied on Hanger kg |  | Distance fromLeft handSupportcm |  | Additional load on thrust hanger | Calculated value ofH |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | W, |  | $a_{1}$ |  |  |  |
| । | $w$, |  | $a_{1}$ |  |  |  |
|  | W, |  | $a_{1}$ |  |  |  |
|  | W, |  |  |  |  |  |
|  | W, |  | $a_{1}$ |  |  |  |
| 2 | rv, |  | $a_{1}$ |  |  |  |
| 2 | W, |  | $a_{1}$ |  |  |  |
|  | II', |  |  |  |  |  |

## Calculation:

## Precautions:

1. Put the weights in thrust hanger very gently without a jerk.
2. Measure the distance of loaded points from left hand support accurately. Perform the experiment away from vibration and other disturbances.

## Results:

1. Find the horizontal thrust for a given set of load experimentally and theoretically.

Experimental value of horizontal thrust, $f$ f...,
Theoretical value of horizontal thrust, $H_{\text {, }}$.

$$
=
$$

$$
=
$$

## Commmts:

## Question:-

- Define two hinge arches?
- What is the main difference in three hinged arch and two hinge arch?
- Three hinge arch structure are- (a) Determinate structure (b) Indeterminate Structure.
- The bending moment of three hinge arch is greater than the bending moment of beam. (a) True (b) False.
- Write the expression for bending moment at any section on the arch. Given- $\mathrm{V}_{\mathrm{A}}=$ Vertical load at point $\mathrm{A}, \mathrm{W}_{1}=$ load from end A at distance $\mathrm{a}, \mathrm{H}=$ horizontal force at point $A, x=e / s$ distance from $A$
- ON the basis of support conditions arches are classified / classified the arches on the basis support conditions.


## Experiment No. 4

## Two hinged arch

Aim: - To study two hinged arch for the horizontal displacement of the roller end for a given system of loading and to compare the same with those obtained analytically.

Apparatus: - Two Hinged Arch Apparatus, Weight’s, Hanger, Dial Gauge, Scale, Verniar Caliper.

Formula: $-\mathrm{H}=5 \mathrm{WL}\left(\mathrm{a}-2 \mathrm{a}^{3}+\mathrm{a}^{4}\right) / 8 \mathrm{r}$

Where,
$\mathrm{W}=\mathrm{Weight}$ applied at end support.
$L=$ Span of two hinged arch.
$r=$ rise of two hinged arch.
$\mathrm{a}=$ dial gauge reading.

## Diagram:-



Theory: - The two hinged arch is a statically indeterminate structure of the first degree.The horizontal thrust is the redundant reaction and is obtained y the use of strain energy methods. Two hinged arch is made determinate by treating it as a simply supported curved beam and horizontal thrust as a redundant reaction. The arch spreads
out under external load. Horizontal thrust is the redundant reaction is obtained by the use of strain energy method.

## Procedure: -

i) Fix the dial gauge to measure the movement of the roller end of the model and keep the lever out of contact.
ii) Place a load of 0.5 kg on the central hanger of the arch to remove any slackness and taking this as the initial position, set the reading on the dial gauge to zero.
iii) Now add 1 kg weights to the hanger and tabulated the horizontal movement of the roller end with increase in the load in steps of 1 kg . Take the reading up to 5 kg load. Dial gauge reading should be noted at the time of unloading also.
iv) Plot a graph between the load and displacement (Theoretical and Experimental) compare. Theoretical values should be computed by using horizontal displacement formula.
v) Now move the lever in contact with 200 gm hanger on ratio $4 / 1$ position with a 1 kg load on the first hanger. Set the initial reading of the dial gauge to zero.
vi) Place additional 5 kg load on the first hanger without shock and observe the dial gauge reading.
vii) Restore the dial gauge reading to zero by adding loads to the lever hanger, say the load is w kg.
viii) The experimental values of the influence line ordinate at the first hanger position shall be 4 w .
ix) Repeat the steps 5 to 8 for all other hanger loading positions and tabulate. Plot the influence line ordinates.
x) Compare the experimental values with those obtained theoretically by using equation (5).

| Sr. <br> No. | Central load <br> (kg) | 0.0 | 1.0 | 2.0 | 3.0 | 4.0 | 5.0 | 6.0 |
| :--- | :---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | Observed <br> horizontal <br> Displacement <br> (mm ) |  |  |  |  |  |  |  |
|  | Calculated <br> horizontal <br> Displacement <br> Eq. (4) |  |  |  |  |  |  |  |

Sample Calculation: - Central load (kg)=, $\qquad$
Observed horizontal Displacement $(\mathrm{mm})=$
Calculated horizontal Displacement $=\mathrm{H}=5 \mathrm{WL}\left(\mathrm{a}-2 \mathrm{a}^{3}+\mathrm{a} 4\right) / 8 \mathrm{r}=$ $\qquad$

Result:-The observed and horizontal displacement is nearly same.
Precaution: - 1.Apply the loads without jerk.
2. Perform the experiment away from vibration and other

## disturbances.Question:-

1. Define two hinged arch.
2. Two hinged arch structure are-
a. Determinate structure
b. indeterminate structure
3. What is the application of two hinged arch?
4. Two hinged arch construction are complicated - TRUE/FLASE
5. The load carrying capacity of two hinge arch is higher than beams- TRUE/FLASE
6. The bending moment of arch is higher than the B.M. of beam- TRUE/FLASE

## Experiment No 5

## Column and buckling

Object: - To study behavior of different types of columns and find Euler's buckling load for each case.

Apparatus: - Column Buckling Apparatus, Weights, Hanger, Dial Gauge, Scale, Verniar caliper.

## Diagram:-


(i) Both encs fixed


(ii) Ore enc fixed other pinned


Theory: -If compressive load is applied on a column, the member may fail either by crushing or by buckling depending on its material, cross section and length. If member is considerably long in comparison to its lateral dimensions it will failby buckling. If a member shows signsof buckling the member leads to failurewith small increase in load. The load at which the member just buckles is called as crushing load. The buckling load, as given by Euler, can be found byusing following expression.

$$
\mathrm{P}=\pi^{2} \mathrm{EI} / \mathrm{le}^{2}
$$

Where,
$\mathrm{E}=$ Modulus of Elasticity $=2 \times 105 \mathrm{~N} / \mathrm{mm}^{2}$ for steel
$\mathrm{I}=$ Least moment of inertia of column section
Le $=$ Effective length of column
Depending on support conditions, four cases may arise. The effective length for each ofwhich are given as:

1. Both ends are fixed le $=\mathrm{L} / 2$
2. One end is fixed and other is pinned le $=\mathrm{L} / \sqrt{ } 2$
3. Both ends are pinned le $=\mathrm{L}$
4. One end is fixed and other is free $\mathrm{le}=2 \mathrm{~L}$

## Procedure: -

i) Pin a graph paper on the wooden board behind the column.
ii) Apply the load at the top of columns increasing gradually. At certain stage of loading the columns shows abnormal deflections and gives the buckling load.
iii) Not the buckling load for each of the four columns.
iv) Trace the deflected shapes of the columns over the paper. Mark the points of change of curvature of the curves and measure the effective or equivalent length for each case separately.
v) Calculate the theoretical effective lengths and thus buckling loads by the expressions given above and compare them with the observed values.

## Observation: -

1) Width of strip (mm) $b=$
2) Thickness of strip $(\mathrm{mm}) t=$
3) Length of strip (mm) $L=$
4) Least moment of inertia
$\mathrm{I}=\mathrm{bt} \mathrm{t}^{3} / 12$

## Observation Table:-

| Sr. <br> No | End condition | Buckling load (P= $\pi^{2}$ EI) |  | Effective Length (mm) |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Theoretical | Observed | Theoretical | Observed |
| $\mathbf{1 .}$ | Both ends fixed |  |  |  |  |


| 2. | One end fixed <br> and other pinned |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 3. | Both ends pinned |  |  |  |  |
| 4. | One end fixed <br> and other free. |  |  |  |  |

Sample Calculation: - Both ends fixed
Euler's buckling load. $=\mathrm{le}^{2}$
Effective Length $(\mathrm{mm})=$.
Result:-The theoretical and experimental Euler's buckling load for each case is foundnearly same.

## Question:-

1. Define buckling?
2. What is the difference $\mathrm{b} / \mathrm{w}$ buckling and twisting?
3. What is the difference $b / w$ column and strut?
4. Define buckling facture?

## Experiment No-6

## Deflections of beam

Aim: - To verify the moment area theorem regarding the slopes and deflections of the beam. Apparatus: - Moment of area theorem apparatus.

## Diagram:-



Fig.-

Theory : - According to moment area theorem

1. The change of slope of the tangents of the elastic curve between any two points ofthe deflected beam is equal to the area of $M / E I$ diagram between these two points.
2. The deflection of any point relative to tangent at any other point is equal to themoment of the area of the M/EI diagram between the two points at which thedeflection is required.Slope at $\mathrm{B}=\mathrm{Y}_{2} /$ bsince the tangent at C is horizontal due to symmetry,
Slope at $\mathrm{B}=$ shaded area $/ \mathrm{EI}=1 / \mathrm{EI}\left[\mathrm{Wa}^{2} / 2+\mathrm{WA}(\mathrm{L} / 2-\mathrm{a})\right]$
Displacement at $B$ with respect to tangent at $C$

$$
\begin{aligned}
& =\left(y_{1}+y_{2}\right)=\text { Moment of shaded area about B } / E I \\
& =1 / E I\left[\mathrm{Wa}^{2} / 2(b+2 / 3 a)+W a(L / 2-a)(b+a / 2+L / 2)\right]
\end{aligned}
$$

## Procedure: -

1. Measure $a, b$ and $L$ of the beam
2. Place the hangers at equal distance from the supports $A$ and load them with equal loads.
3. Measure the deflection by dial gauges at the end $B\left(y_{2}\right)$ and at the center $C$ ( $y_{1}$ )
4. Repeat the above steps for different loads.

## Observation Table:-

Length of main span, $\mathrm{L}(\mathrm{cm})=$
Length of overhang on each side, $\mathrm{a}(\mathrm{cm})=$
Modulus of elasticity, $\mathrm{E}\left(\mathrm{kg} / \mathrm{cm}^{2}\right)=2 \times 10^{6}$

| Sl. No. | Load at each <br> Hanger (kg) | Central <br> Deflection <br> $\mathrm{Y}_{1}(\mathrm{~cm})$ | Deflection at <br> Free end $\mathrm{y}_{2}$ <br> $(\mathrm{~cm})$ | Slope at B <br> $\mathrm{Y}_{2} / \mathrm{b}$ | Deflection at <br> C=Deflection atB (y1) |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |

## Calculation:-

1. Calculate the slope at B as $\mathrm{y}_{2} / \mathrm{b}$ (measured value).
2. Compute slope and deflection at B theoretically from B.M.D. and compare withexperimental values.
3. Deflection at $\mathrm{C}=\mathrm{y}_{1}$ (measured value).
4. Deflection at $\mathrm{C}=$ Average calculated value

Result: -The slope and deflection obtained is close to the slope and deflection obtained by using moment area method.

## Precaution:-

1. Apply the concentration loads without jerks.
2. Measures the deflection only when the beam attains ion.
3. Measures the deflection very carefully and accurately.
4. Check the accuracy and least count of dial gauges used for measuring deflections.

## Experiment:-7

## Deflection of curved members

## Aim:

To determine the elastic displacement of the curved members experimentally and verify the same with the analytical results.

## Apparatus:

Curved beam apparatus with four different types of configurations, weights, scale, dial gauges and Vernier Caliper.

## Theory:

The elastic displacements of a curved member can be determined using Castigliano's first theorem which states that "The partial derivative of the strain energy with respect to any force gives the displacement of the point of its application in the direction of the force."

The total strain energy of any structure is determined in terms of the entire load with their actual values and a fictitious load $P$ applied at the point at which the deflection is required and it is acting in the same direction in which the deflection is required. In case no external load is acting at the joint in the direction desired, a fictitious load is applied in that direction and forces in all the members are worked out. After partial differentiation with respect to P , zero is substituted for the fictitious load P (or if P is not fictitious its actual value is substituted). Thus the result is the required deflection.
a. Quadrant of a circle

The curved beam is fixed at the point A and is free at point B. The concentrated load, P is applied at the free end.

Vertical displacement at point B along the line of action of the load $\left(\delta_{V B}\right)$

$$
\delta_{V B}=\frac{\pi P R^{3}}{4 E I}
$$

where, $R \quad=$ Radius of the quadrant,


E = Young's modulus of the material of the beam
$=2 \times 10^{5} \mathrm{~N} / \mathrm{mm}^{2}$

- Moment of Inertia ofthe cross section of the curved member

$$
=-\frac{b d!}{12}
$$

I lorizental displacement at point $\mathrm{O}\left(5_{118)}\right.$

$$
\sigma_{,, .}=P E
$$

## b. Quadrant of a circle with a straigh: leg.

The member is a quadrant from point A to Band then straight line from B to C
Vertical displacement at point C along the line of action of the load ( $6,-$.

$$
\underset{r e}{\mathrm{a}}=\underset{E l}{ }
$$

$J$ Jorizontal displacemem at point $\mathrm{B}\left(\mathrm{O}_{118}\right)$

$$
6, c=--\frac{P R}{2 E I}\left[; r y^{2}+\frac{n R^{\prime}}{8}+4 y R\right]
$$


c. Semksrcte with straig/ll arm

Vertical displacement at point $C$ along the line of action of the load ( $0_{, 9,9}$ )

$$
\sigma_{\mathrm{It}^{\prime \prime}}:: \frac{\mathrm{JE} / \mathrm{E}^{+}+-\frac{P R[ }{E l} n R^{\prime}+.!L_{2}^{2]}}{2}
$$



Horizontal displacement at point $\mathrm{B}\left(\mathrm{O}_{118}\right)$

$$
\sigma_{I I C}=-P R_{£ /}^{\prime}\left[n R+l \cdot{ }_{2}\right]
$$



## d. Circle

Vertical displacement at point C along the line of action of the load ( $0,-$.

$$
a_{i}=-\frac{n P R^{\prime}}{21 \mathrm{E} /}
$$



## Procedure:

,. Place a load of 0.5 kg on the hanger to activate the member and treat this as the initial position for measuring deflection.
11. FLx the dial gauges for measuring horizontal and vertical deflections.
111. Place the additional loads at an increment of 0.5 kg and tabulate the dial gauge readings against the applied loads.

## Observation:

a. Quadrant

| $\begin{gathered} \text { S. } \\ \text { No } \end{gathered}$ | $\underset{\mathrm{kg}}{\mathrm{Load}}$ | Vertical deflection (mm) |  |  |  | Horizontal deflection (mm) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dial Gauge reading |  |  | Theoretical | Dial Gauge reading |  |  | Theoretical |
|  |  | Initial | Final | Actual | $\delta_{V B}$ | Initial | Final | Actual | $\delta_{H B}$ |
| 1 | 0.5 |  |  |  |  |  |  |  |  |
| 2 | 1.0 |  |  |  |  |  |  |  |  |
| 3 | 1.5 |  |  |  |  |  |  |  |  |
| 4 | 2.0 |  |  |  |  |  |  |  |  |

b. Quadrant with straight leg

| $\begin{aligned} & \text { S. } \\ & \text { No } \end{aligned}$ | $\underset{\mathrm{kg}}{\mathrm{Load}}$ | Vertical deflection (mm) |  |  |  | Horizontal deflection (mm) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dial Gauge reading |  |  | Theoretical | Dial Gauge reading |  |  | Theoretical |
|  |  | Initial | Final | Actual | $\delta_{V B}$ | Initial | Final | Actual | $\delta_{\text {HB }}$ |
| 1 | 0.5 |  |  |  |  |  |  |  |  |
| 2 | 1.0 |  |  |  |  |  |  |  |  |
| 3 | 1.5 |  |  |  |  |  |  |  |  |
| 4 | 2.0 |  |  |  |  |  |  |  |  |

c. Semicircle with straight arm

| $\begin{gathered} \text { S. } \\ \text { No } \end{gathered}$ | $\underset{\mathrm{kg}}{\text { Load }}$ | Vertical deflection (mm) |  |  |  | Horizontal deflection (mm) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dial Gauge reading |  |  | Theoretical | Dial Gauge reading |  |  | Theoretical |
|  |  | Initial | Final | Actual | $\delta_{V B}$ | Initial | Final | Actual | $\delta_{H B}$ |
| 1 | 0.5 |  |  |  |  |  |  |  |  |
| 2 | 1.0 |  |  |  |  |  |  |  |  |
| 3 | 1.5 |  |  |  |  |  |  |  |  |
| 4 | 2.0 |  |  |  |  |  |  |  |  |

d. Circle

| $\begin{aligned} & \text { S. } \\ & \text { No } \end{aligned}$ | $\begin{array}{\|c} \mathrm{Load} \\ \mathrm{~kg} \end{array}$ | Vertical deflection (mm) |  |  |  | Horizontal deflection (mm) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dial Gauge reading |  |  | Theoretical | Dial Gauge reading |  |  | Theoretical |
|  |  | Initial | Final | Actual | $\delta_{V B}$ | Initial | Final | Actual | $\delta_{H B}$ |
| 1 | 1 |  |  |  |  |  |  |  |  |
| 2 | 2 |  |  |  |  |  |  |  |  |
| 3 | 3 |  |  |  |  |  |  |  |  |
| 4 | 4 |  |  |  |  |  |  |  |  |

Dimensions of the beam

| $\begin{gathered} \hline \text { S. } \\ \text { No } \\ \hline \end{gathered}$ | Configuration | Quadrant | Quadrant with straight leg | Semicircle with straight arm | Circle |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Main Scale |  |  |  |  |
|  | Vernier Scale |  |  |  |  |
|  | Total |  |  |  |  |
| 2 | Radius |  |  |  |  |
| 3 | Arm/leg length | - |  |  | - |

## Calculation:

## Precautions:

i. Apply the loads gently
ii. Measure the displacements very accurately

## Results:

1. Plot the graph between load and deflection for each case to show that the structure remains within the elastic limit.
2. Vertical deflection in $\mathrm{mm}, \delta_{V B}$

| S. | Case (a) |  | Case (b) |  | Case (c) |  | Case (d) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| No | Exp. | Calc. | Exp. | Calc. | Exp. | Calc. | Exp. | Calc. |
| 1 |  |  |  |  |  |  |  |  |
| 2 |  |  |  |  |  |  |  |  |
| 3 |  |  |  |  |  |  |  |  |
| 4 |  |  |  |  |  |  |  |  |

3. Horizontal deflection, $\delta_{t r s}$

| S. | Case (a) |  | Case (b) |  | Case (c) |  | Case (d) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Exp. | Calc. | Exp. | Calc. | Exp. | Calc. | Exp. | Calc. |
| 1 |  |  |  |  |  |  |  |  |
| 2 |  |  |  |  |  |  |  |  |
| 3 |  |  |  |  |  |  |  |  |
| 4 |  |  |  |  |  |  |  |  |

Comments:

## Experiment:- 8

## Analysis of Redundant frame

## Aim:

To study the behaviour redundant frame subjected to coplanar force experimentally and to verify the horizontal and vertical displacements obtained from the experiment with the analytical results.

## Apparatus:

Three bar suspension system, weights, scale, dial gauge, etc.

## Theory:

The diagram of the apparatus is shown in the figure below.


The horizontal $(U)$ and the vertical $(V)$ displacements of the point D is calculated as follows

$$
\begin{aligned}
U & =\frac{W}{L_{3}} \times \frac{\left(N_{1} a-N_{2} b\right)}{N_{1} N_{2}(a+b)^{2}+N_{3}\left(n_{1} a^{2}+n_{2} b^{2}\right)} \\
V & =\frac{W}{L_{3}^{2}} \times \frac{\left(N_{1} a^{2}+N_{2} b^{2}\right)}{N_{1} N_{2}(a+b)^{2}+N_{3}\left(n_{1} a^{2}+n_{2} b^{2}\right)}
\end{aligned}
$$

where,

$$
\begin{array}{ll}
N_{1} & =\frac{A_{1} E_{1}}{L_{1}} \times \frac{1}{L_{1}^{2}} \\
N_{2} & =\frac{A_{2} E_{2}}{L_{2}} \times \frac{1}{L_{2}^{2}} \\
N_{2} & =\frac{A_{3} E_{3}}{L_{3}} \times \frac{1}{L_{3}^{2}} \\
L_{1} & =\text { Length of the member AD } \\
L_{2} & =\text { Length of the member BD } \\
L_{2} & =\text { Length of the member CD } \\
a & =\text { Distance between A and B } \\
b & =\text { Distance between A and B } \\
W & =\text { Applied load at D }
\end{array}
$$

The tensile force in the members are calculated as follows

$$
\begin{gathered}
T_{1}=\frac{\left(L_{3} V-a U\right) A_{1} E_{1}}{L_{1}^{2}} \\
T_{2}=\frac{\left(L_{3} V+b U\right) A_{2} E_{2}}{L_{2}^{2}} \\
T_{3}=\frac{\left(L_{3} V\right) A_{3} E_{3}}{L_{3}^{2}}
\end{gathered}
$$

where,
$T_{1} \quad=$ Tension force in member AD
$T_{2} \quad=$ Tension force in member BD
$T_{3} \quad=$ Tension force in member CD
The expression $\frac{A E}{L}$ represents the axial stiffness of the structure. It denotes the force required to produce unit deformation. This value can be calculated by finding the slope from load vs. deflection graph plotted for each spring.

## Procedure:

1. Isolate each spring, apply load and measure the deflection and tabulate it.
2. Draw a graph between load ( $y$-axis) and deflection ( $x$-axis) for each spring and find the slope. The value of the slope corresponds to the stiffness of each spring.
3. Connect the lower end of the spring to make a redundant frame.
4. Apply load at increments and note down the horizontal and vertical displacements and the reading in each spring.
5. Calculate the tension force in each spring, horizontal and vertical displacement of point D and compare with the experimental results.

## Observation:

| Length of member AD | $=$ |
| :--- | :--- |
| Length of member BO | $=$ |
| Length of member CD | $=$ |
| Distance a | $=$ |
| Distance $b$ | $=$ |
| Young's Modulus, $E$ | $=$ |


| Load, kg | Deflection in member, mm |  |  |
| :---: | :---: | :---: | :---: |
|  | AD | BO | CD |
| 1.0 |  |  |  |
| 2.0 |  |  |  |
| 3.0 |  |  |  |
| 4.0 |  |  |  |


|  | load, kg | Deflection, mm |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Vertical. $V$ | Surina AD | Soring BO | Spring Cl) |  |
| 1.0 |  |  |  |  |  |  |
| 2.0 |  |  |  |  |  |  |
| 3.0 |  |  |  |  |  |  |
| 4.0 |  |  |  |  |  |  |

## Calculation:

## Precaution:

i. Calculate the spring stiffnesses carefully

11, Measure the distances $\mathrm{AD}, \mathrm{BD}, \mathrm{CD}, a$ and $b$ accurately.
iii. Tap the dial gauges before taking a reading for vertical and horizontal displacements.

## Results:

| Load, kg | Experimental |  |  |  |  | Analytical |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Deflection, mm |  | Force, N |  |  | Deflection, |  | Force, N |  |  |
|  | $u$ | $v$ | T, | T2 | Ti | $U$ | $v$ | T, | 12 | Ti |
| 1.0 |  |  |  |  |  |  |  |  |  |  |
| 2.0 |  |  |  |  |  |  |  |  |  |  |
| 3.0 |  |  |  |  |  |  |  |  |  |  |
| 4.0 |  |  |  |  |  |  |  |  |  |  |

## Comments:

$\square$

